

# Projeto Integrado

## Passarela para animais silvestres

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**UNifeob**

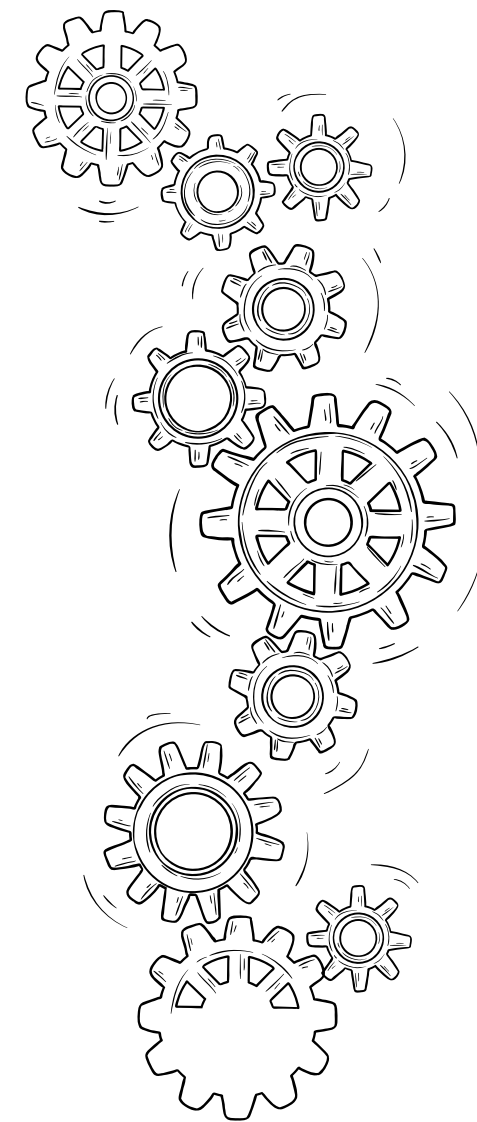
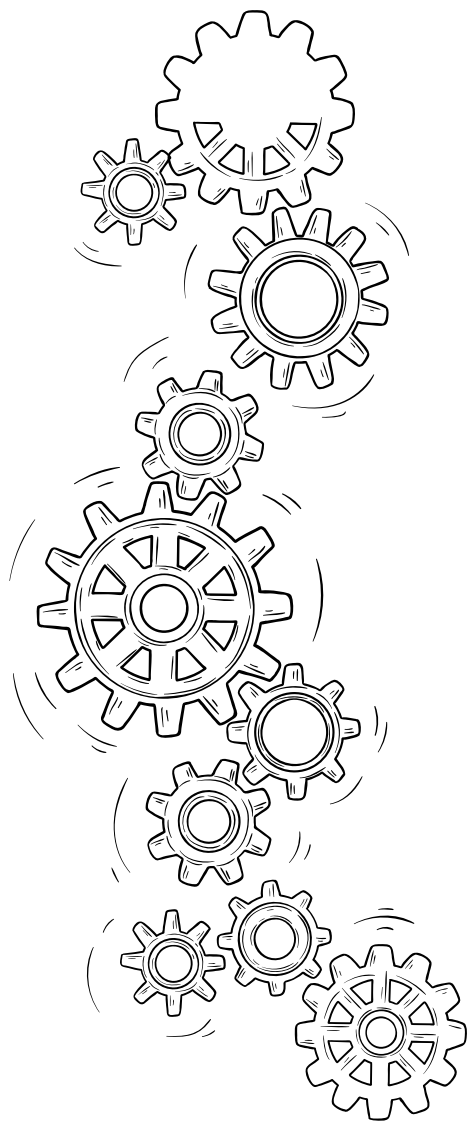
# Memorial de Cálculo

Aço Utilizado Guerdal:

Perfil I:.....76.2mm x 6.38kg/m. Perfil

Cantoneira:.....50.0mm x 2.36kg/m. Perfil

H:.....150mm x 13.0kg/m.



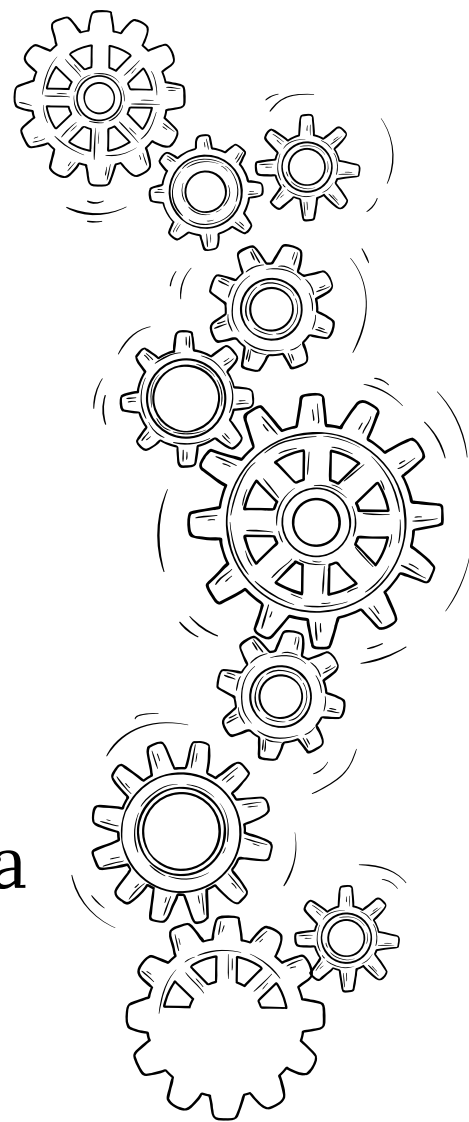
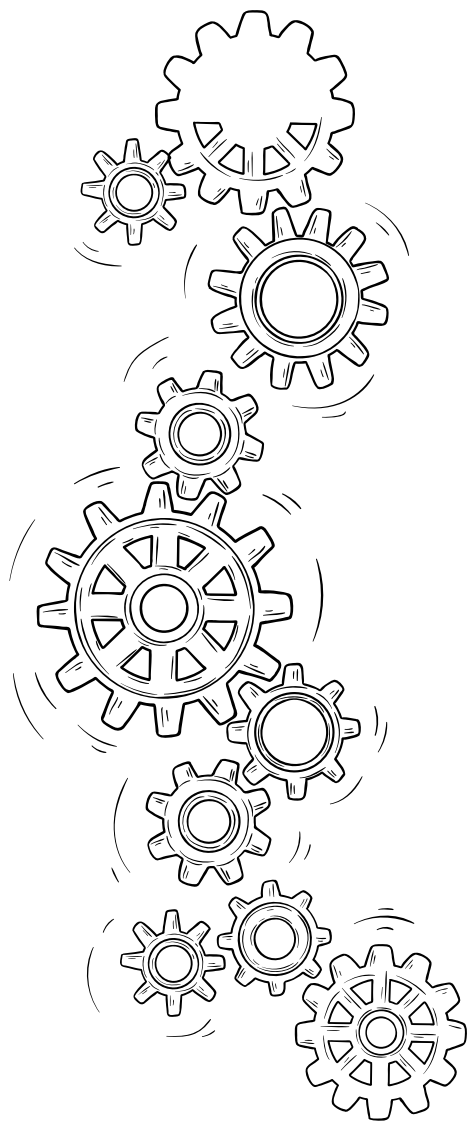
# Memorial de Cálculo

ATM - A36 Começa a deformar com cerca de 25 kg por  $\text{mm}^2$ :  $(f_y) = 25\text{Kg}/\text{mm}^2$ .

Modo de elasticidade do aço:  $(E) = 200\text{GPa} = 20.000\text{KN}/\text{cm}^2$ .

Condição de apoio definida pela tabela:  $(K) = 1$ .

Área da seção transversal, tabela Guerdal:  $(A) = \text{Perfil I } 23.6\text{cm}^2$ , Perfil Cantoneira  $29.10\text{cm}^2$ , Perfil H  $16.6\text{cm}^2$ .



# Memorial de Cálculo



Comprimento da barra: ( L ) = Perfil I 1.95m, perfil Cantoneira 2.18m, 1.95m, Perfil H 2.93m.

Raio de giração mínimo: ( i mini ) = Perfil I 1.92, Perfil Cantoneira 1.57, Perfil H 2.60.

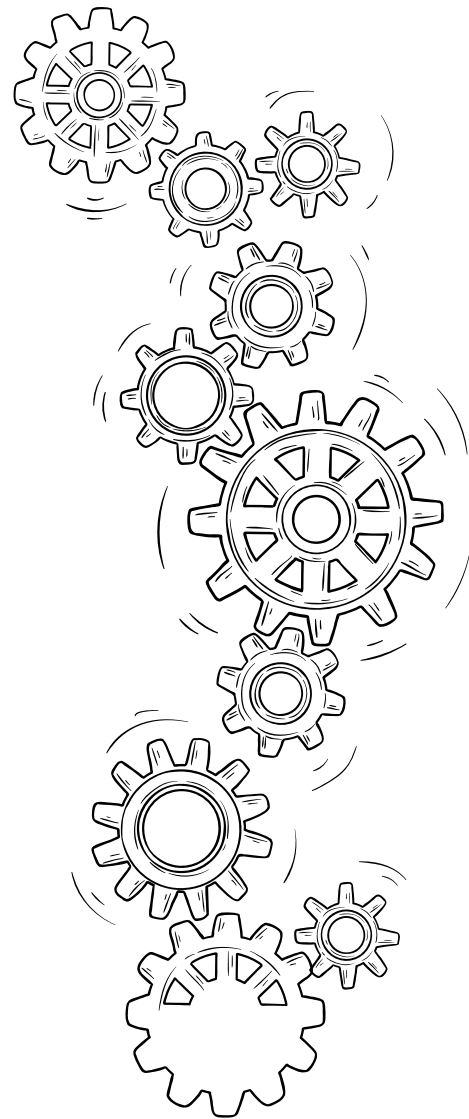
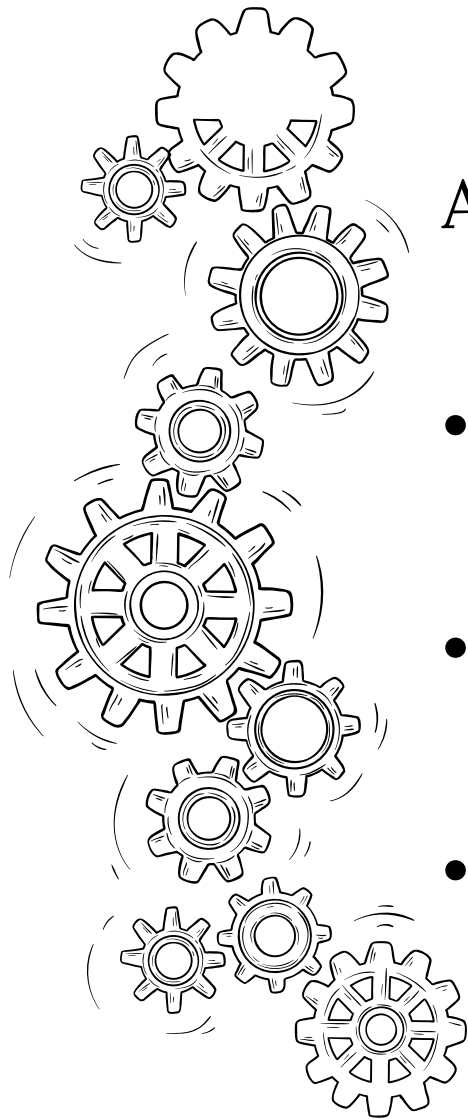
Momento inercia mínimo: ( I mini ) Perfil I 75.7cm<sup>4</sup>, Perfil cantoneira 7.15cm<sup>4</sup>



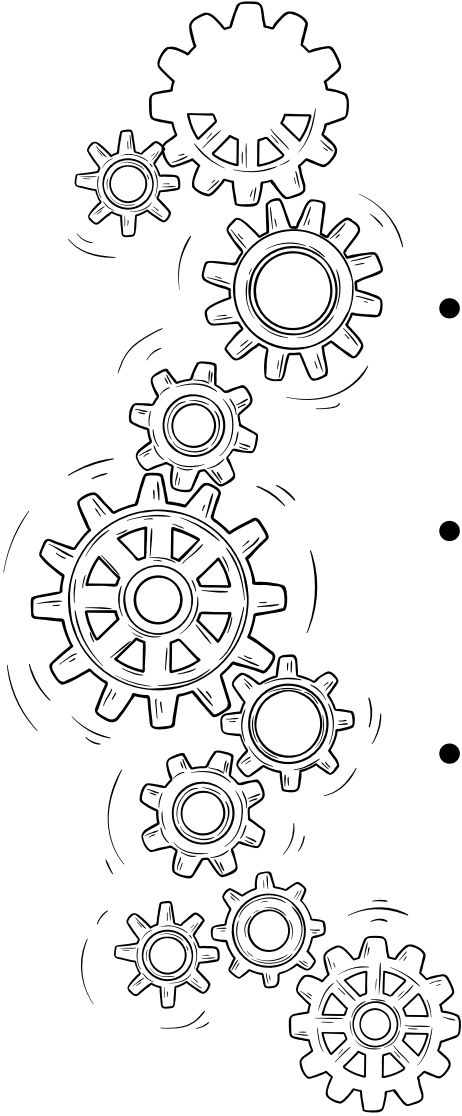
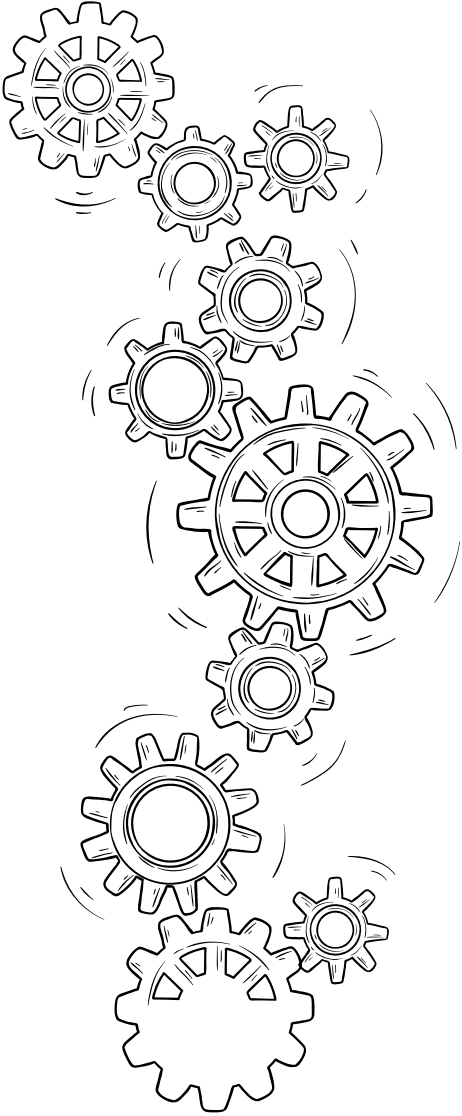
# Memorial de Cálculo

Análise de carga perfil I 76.2mm x 6.38kg/m. Comprimento 1.95m.

- Esbeltez.  $k \cdot L \hat{i} \hat{m}i\hat{n}i 1 \times 195 = 195\text{cm}. 1 \times 195 / 1.92 = 101.53.$
- Flambagem.  $\Pi^2 \cdot 20000 (1 \cdot 195 / 1.92)^2 3.14^2 \times 20.000 / (1 \times 195 / 1.92)^2 = 19.13\text{kg/cm}^2.$
- Compressão.  $\Pi^2 \cdot E \cdot I L^2 3.14^2 \times 20.000 \times 45.60 / 195^2 = 236.71\text{KN}.$



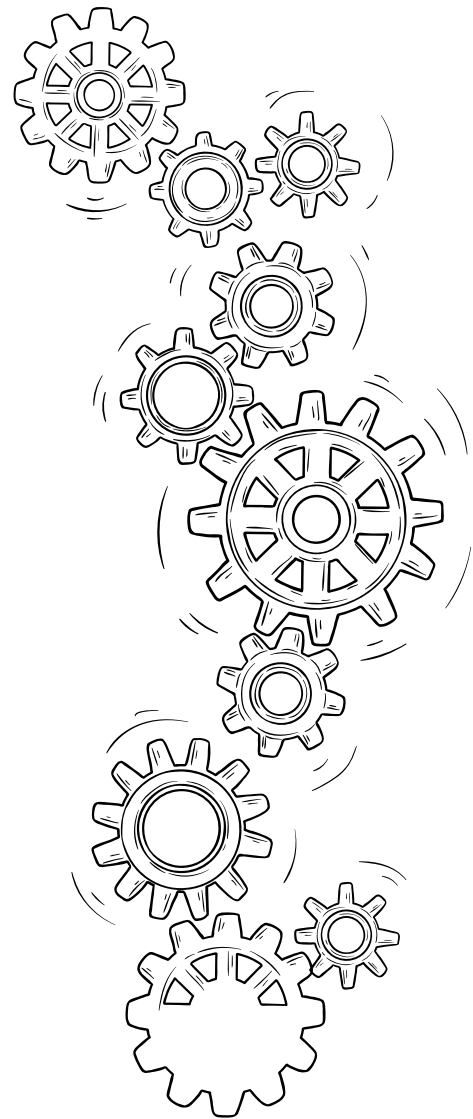
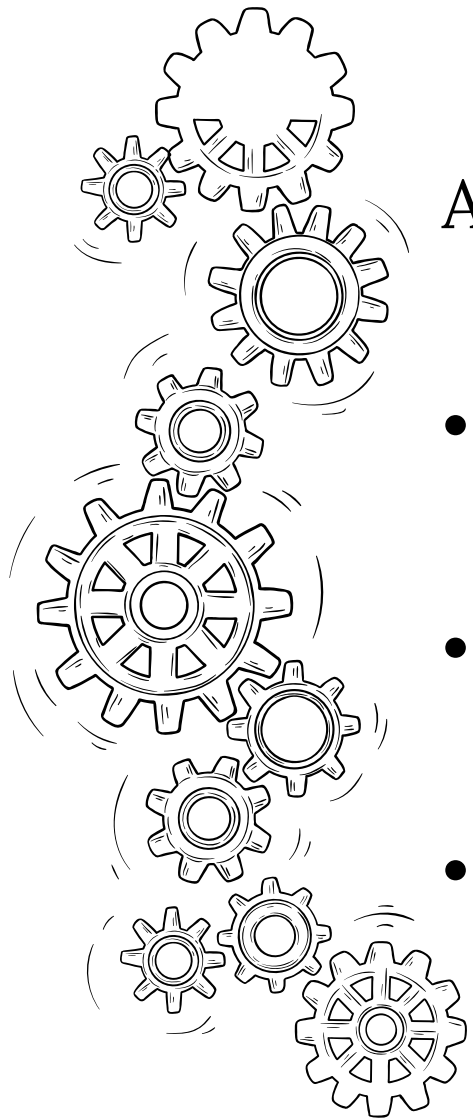
# Memorial de Cálculo

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- Índice de esbeltez.  $\sqrt{A \cdot f_y} \lambda \{12.32 \times 25 / 101.56\} = 1.74.$
  - Curva de Flambagem.  $0,877 \lambda \sigma^2 0.877 / 1.74^2 = 0.29$
  - Força de resistência a compressão.  $x \cdot A \cdot f_y y 0.29 \times 12.32 \times 25 / 1.1 = 81.2\text{KN}.$
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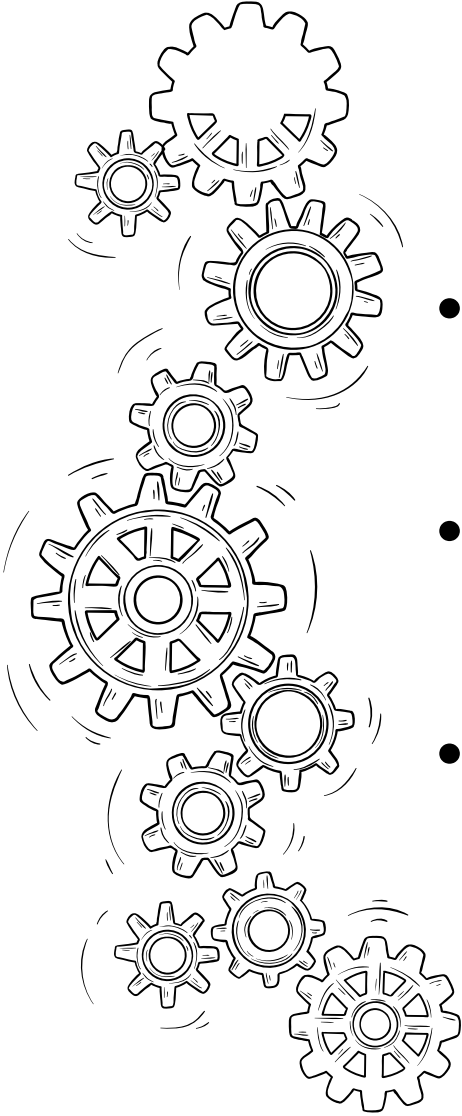
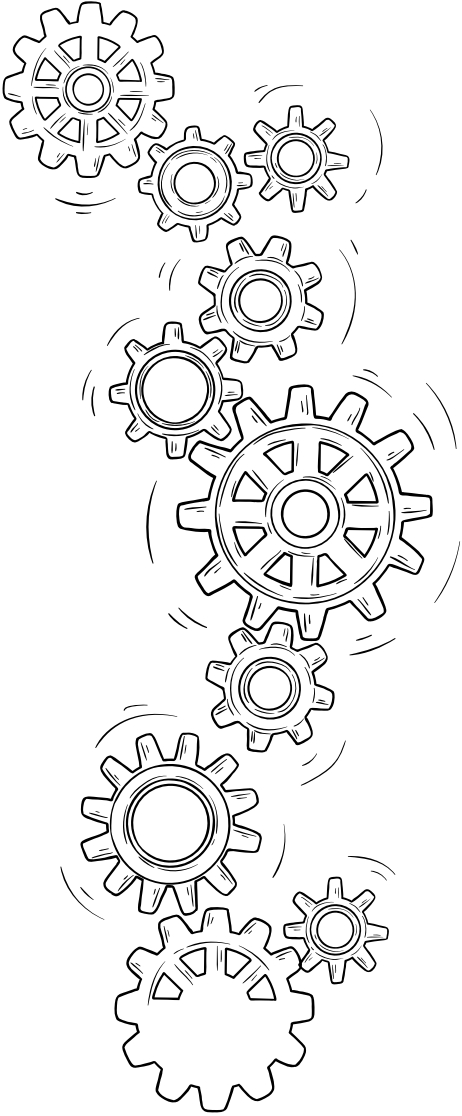
# Memorial de Cálculo

Análise de carga Perfil Cantoneiro 50.0mm x 2.36kg/m. Comprimento 2.18m.

- Esbeltez.  $k \cdot L \dot{\imath} \dot{m}i\dot{n}i$   $1 \times 218 = 218\text{cm. } 1 \times 218 / 1.57 = 138.85.$
- Flambagem  $\Pi^2 \cdot 20000 (1 \cdot 218/1.57)^2 3.14^2 \times 20.000 / (1 \times 218 / 1.57)^2 = 10.4\text{kg/cm}^2.$
- Compressão.  $\Pi^2 \cdot E \cdot I L^2 3.14^2 \times 20.000 \times 7.15 / 218^2 = 29.70\text{KN}.$



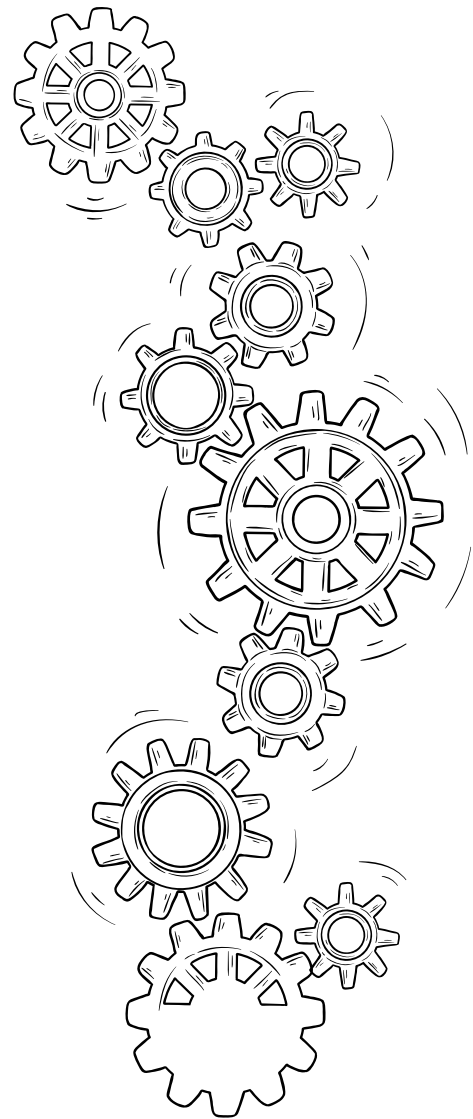
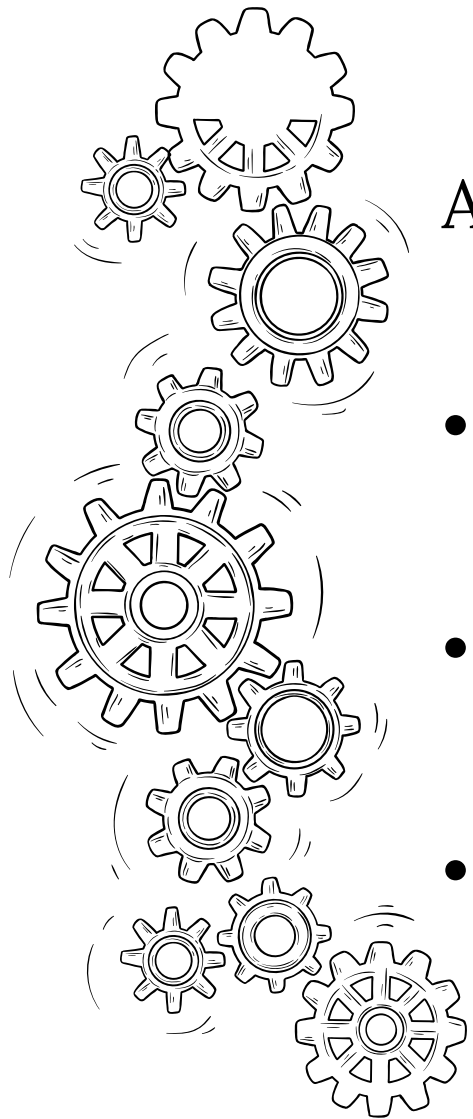
# Memorial de Cálculo

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- Índice de esbeltez.  $\sqrt{A \cdot f_y} \lambda \{3.10 \times 25 / 138.85\} = 0.79.$
  - Curva de Flambagem.  $0.6480.792 \ 0.658^{0.79^2} = 0.79.$
  - Força de resistência a compressão.  $x \cdot A \cdot f_y y \ 0.79 \times 3.1 \times 25 / 1.1 = 55.65\text{KN}.$
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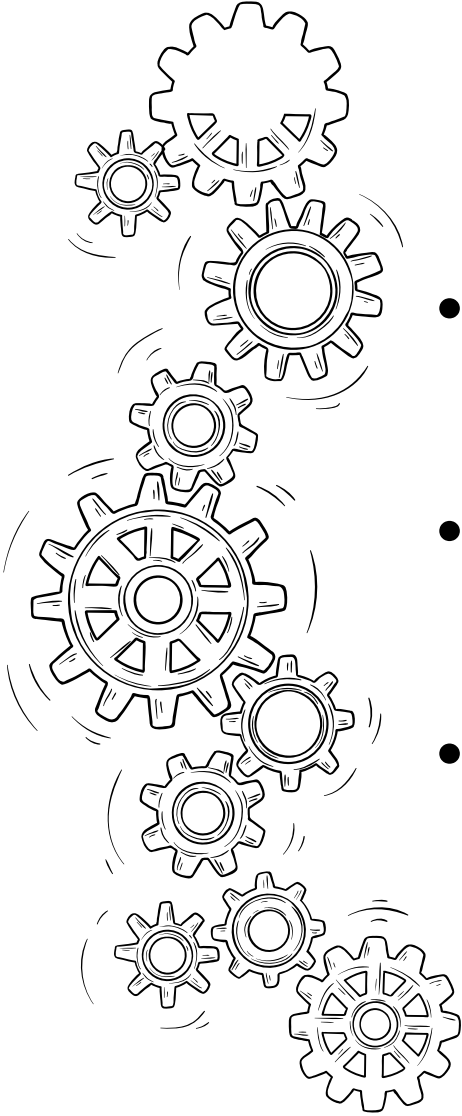
# Memorial de Cálculo

Análise de carga Perfil H 150mm x 13.0kg/m. Comprimento 2.93m.

- Esbeltez  $k \cdot L \dot{i} \dot{m}i\dot{n}i$   $1 \times 2.93 = 293\text{cm}$   $1 \times 293 / 2.22 = 131.98$
- Flambagem  $\Pi^2 \cdot 20000 (1 \cdot 293/2 \cdot 22)^2 3.14^2 \times 20.000 / (1 \times 293 / 2.22)^2 = 11.33\text{kg/cm}^2$
- Compressão  $\Pi^2 \cdot E \cdot I L^2 3.14^2 \times 20.000 \times 82 / 293^2 = 188.54\text{KN}$



# Memorial de Cálculo



- Índice de esbeltez  $\sqrt{A \cdot f_y} \lambda \{16.6 \times 25 / 131.98\} = 1.77$

- Flambagem  $0,877 \lambda \sigma^2 0.877 / 1.77^2 = 0.29$

- Força de resistência a compressão  $x \cdot A \cdot f_y y 0.29 \times 16.6 \times 25 / 1.1 = 109.4.2\text{KN}$



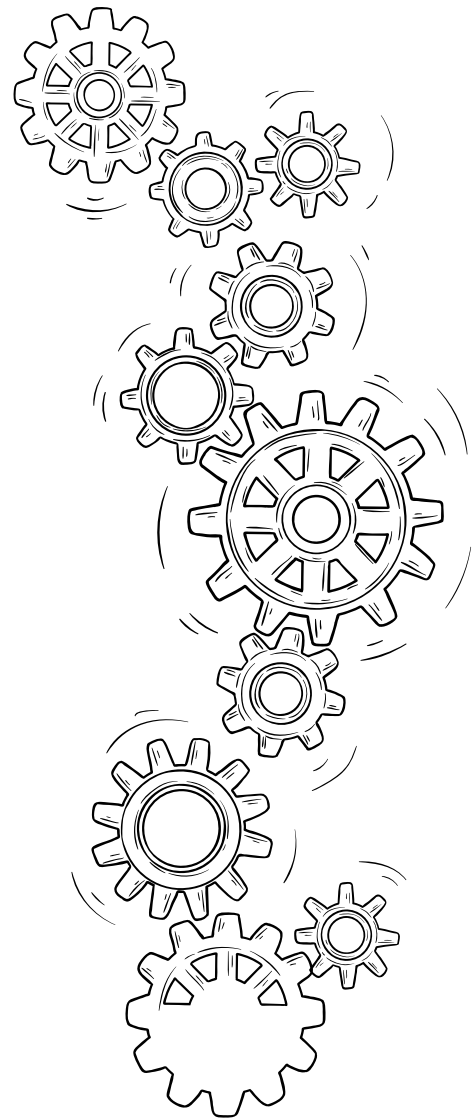
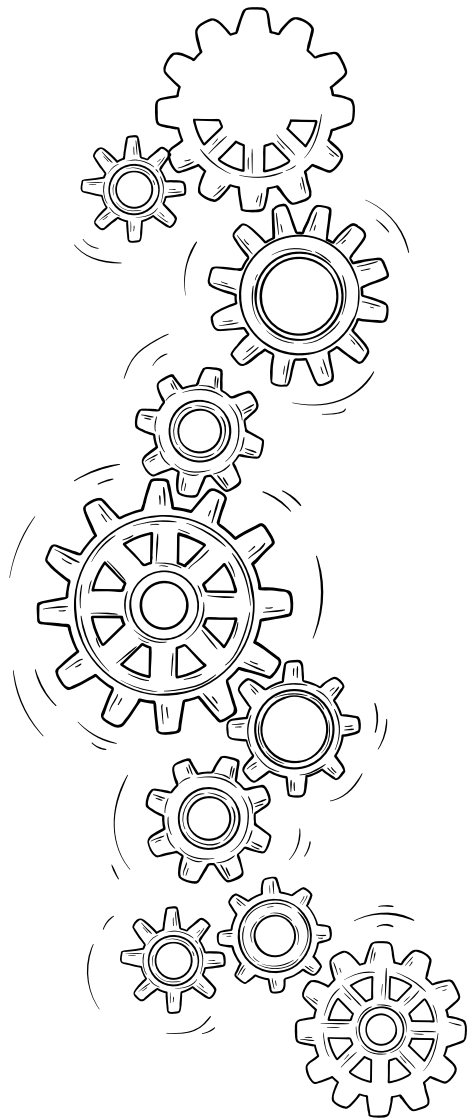
# Memorial de Cálculo

Área de Tributação  $1.95\text{m} \times 1.95 / 2 = 1.90\text{m}^2$ .

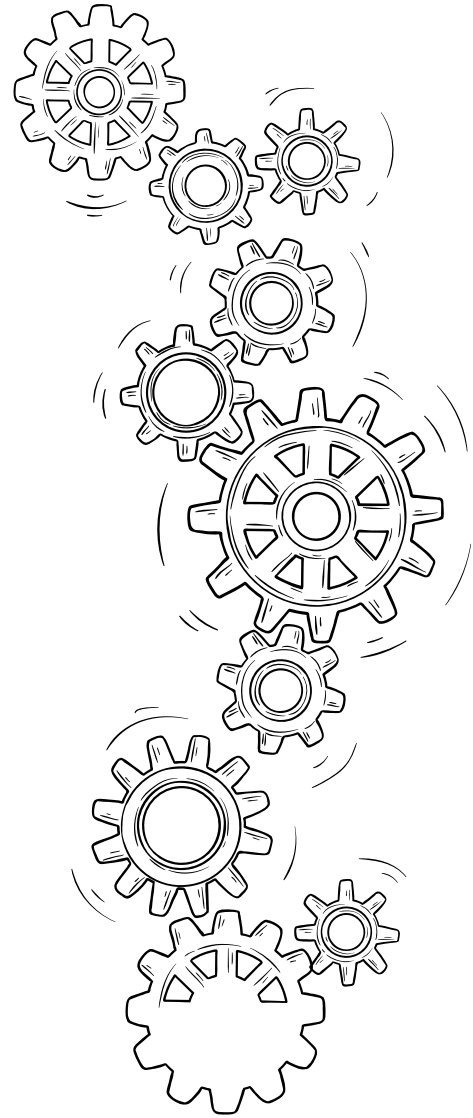
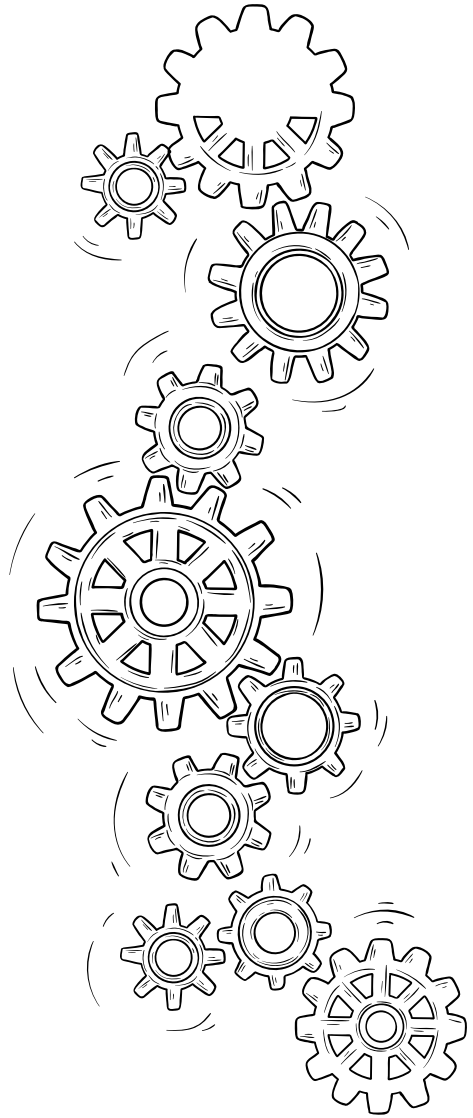
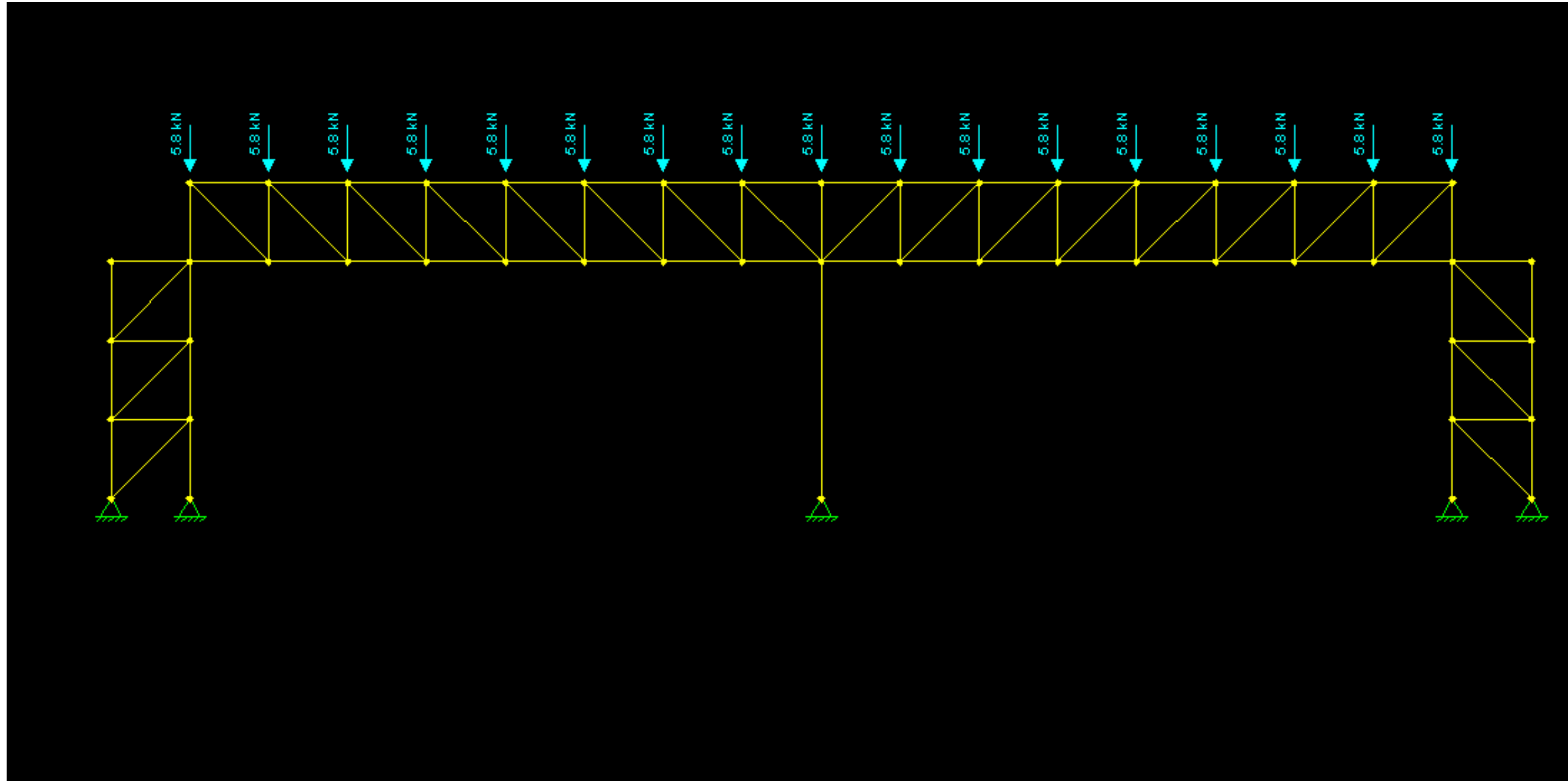
Carga Permanente.  $C_p = 30.19 + 37.35 + 20 = 87.54\text{kg}/\text{m}^2$   $C_p = 87.54 \times 1.90 = 166.32\text{kg}/\text{m}^2$   
 $166.32\text{kg}/\text{m}^2 = 1.6\text{kN}$  }

Carga variável.  $C_q = 200\text{kg}/\text{m}^2 \times 1.90\text{m}^2 = 380\text{kg}/\text{m}^2 = 3.72\text{kN}$   $C_q = 3.72 \times (30\%) 1.11 =$   
 $4.12\text{kN}$   $C_p + C_q 1.6 + 4.12 = 5.75\text{kN}$

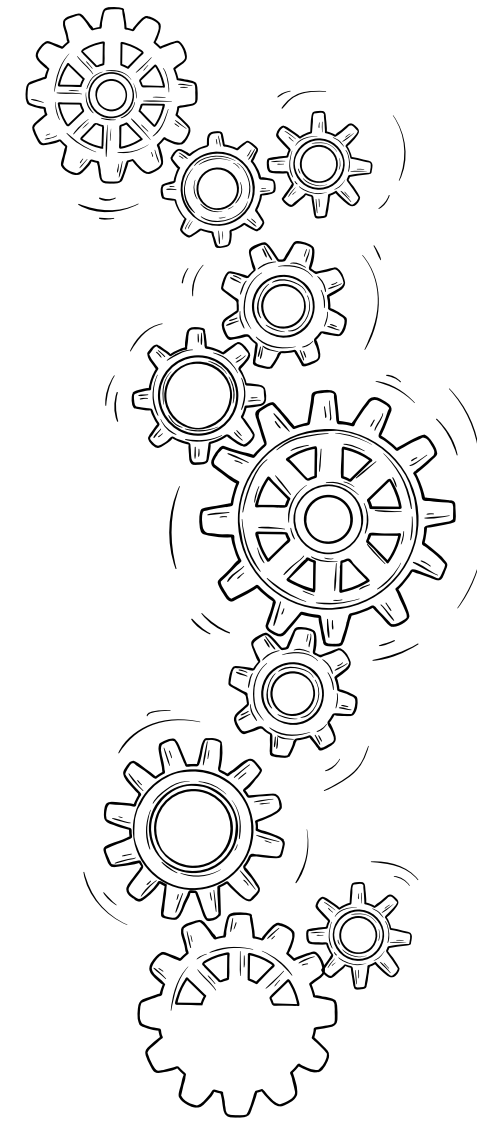
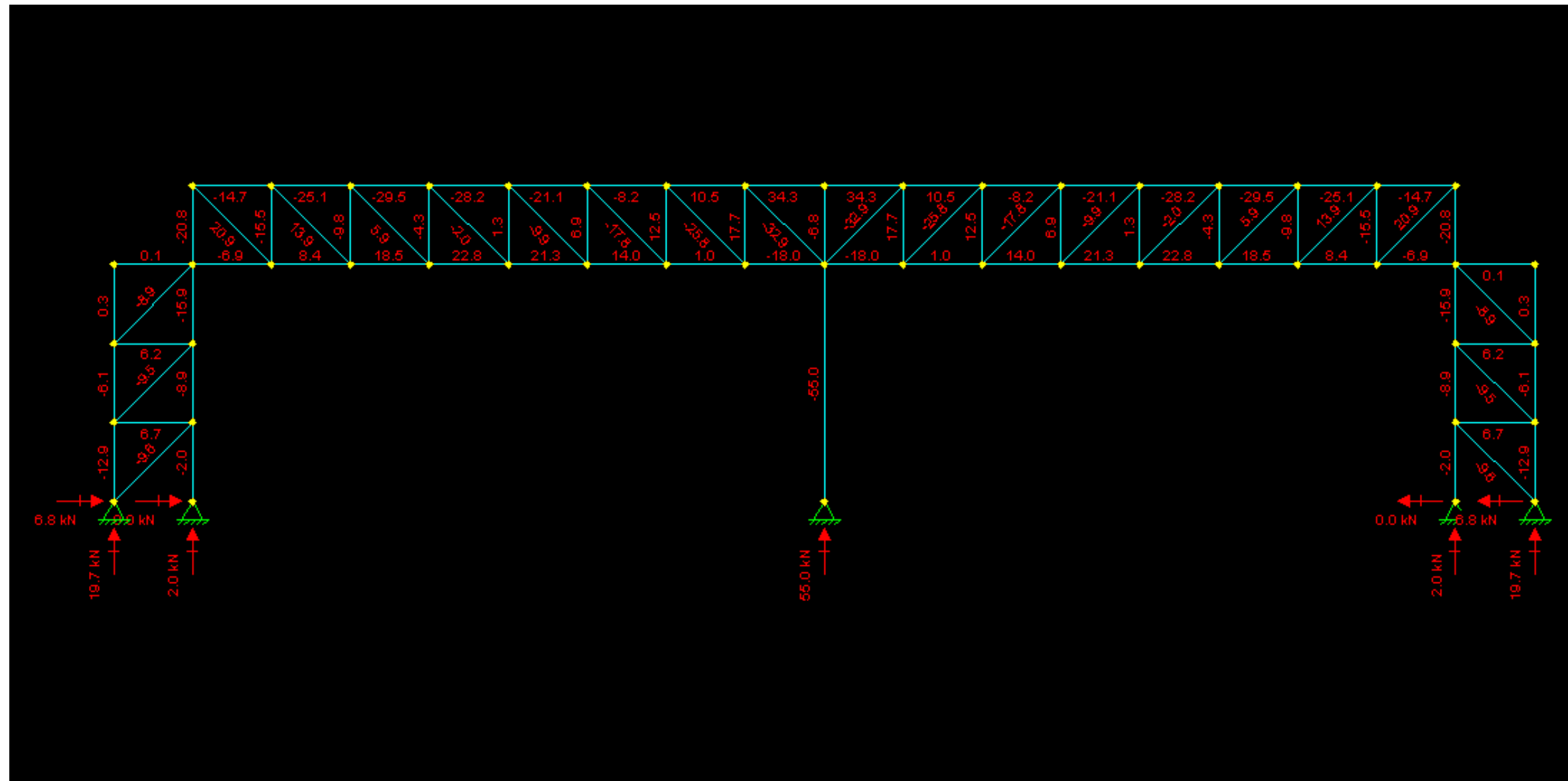
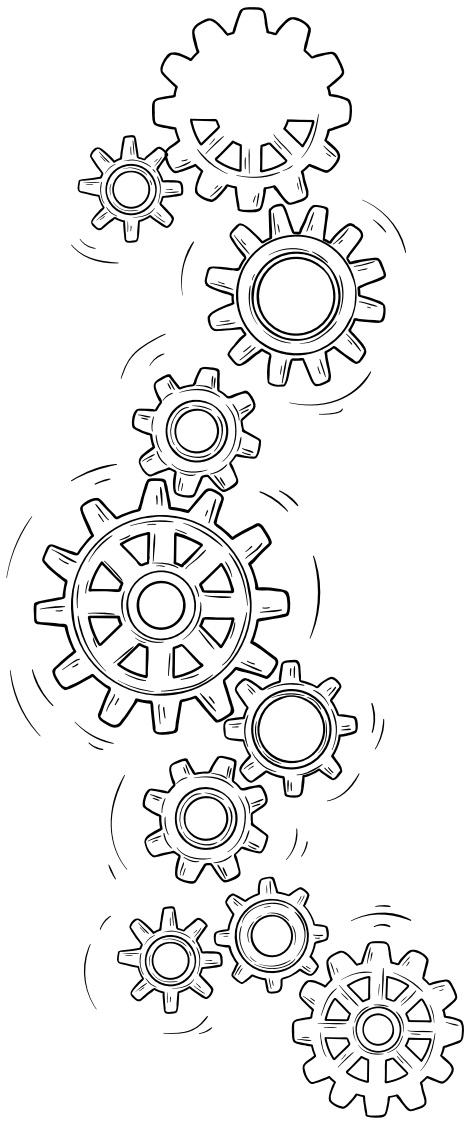
Área de Tributária em cada nó da Rampa. Área;  $6.17 \times 0.975 = 6.01$   $CP 79,72\text{kg}/\text{m}^2$   $Cq$   
 $200\text{kg}/\text{m}^2 + 30\text{kg}/\text{m}^2$   $CP + Cq = 309.72$   $309.72 \times 6.01 = 1863,19$   $18.27\text{KN}$



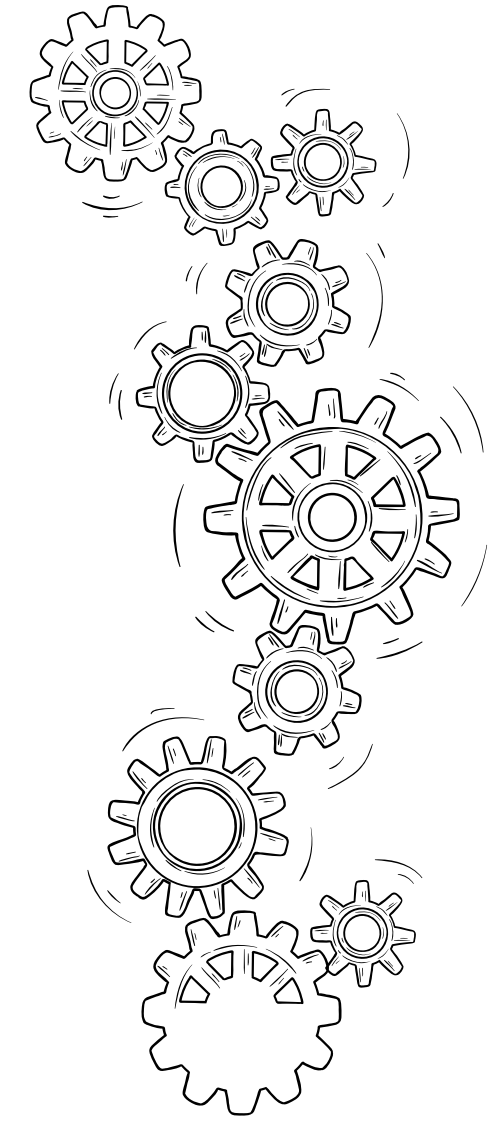
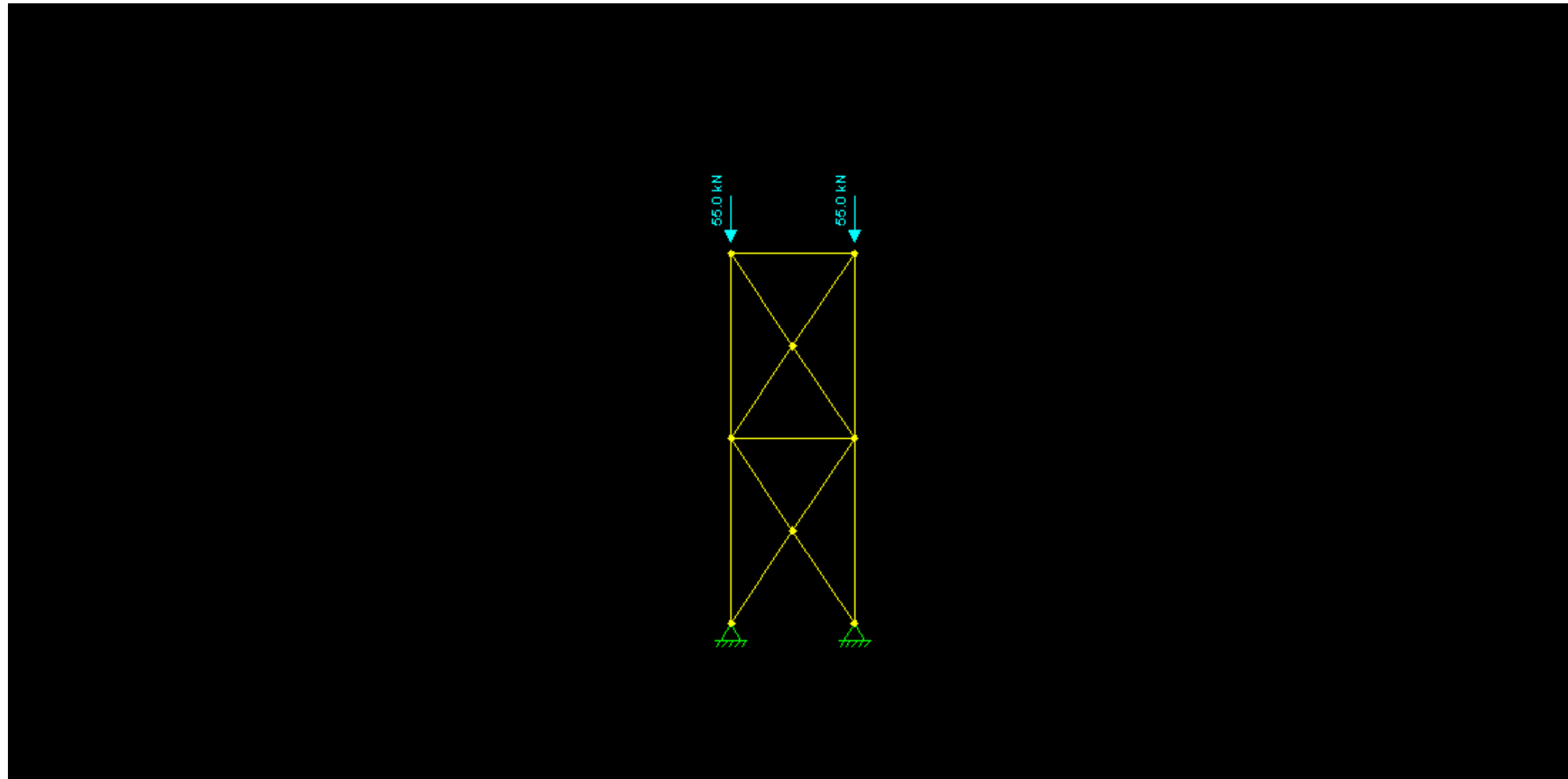
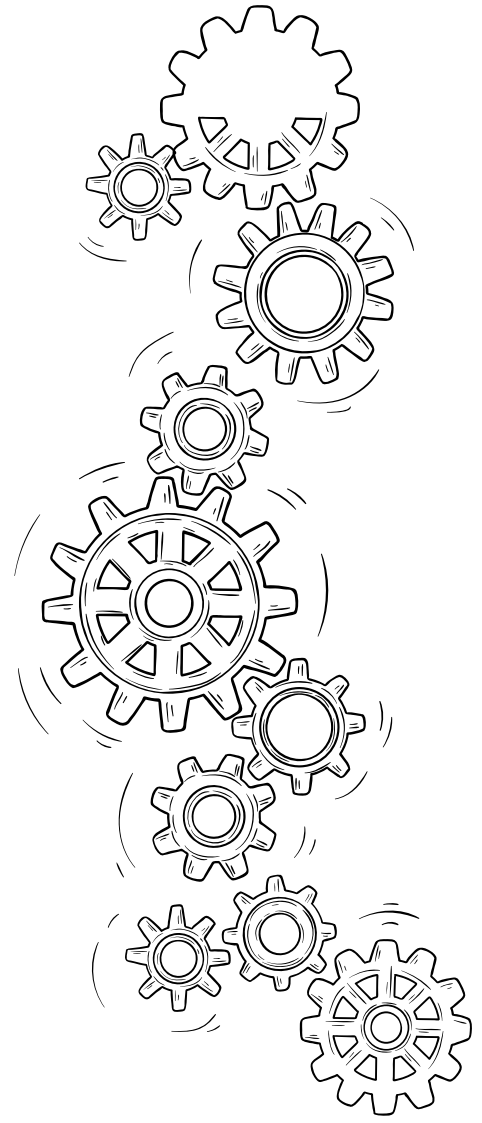
# Imagens



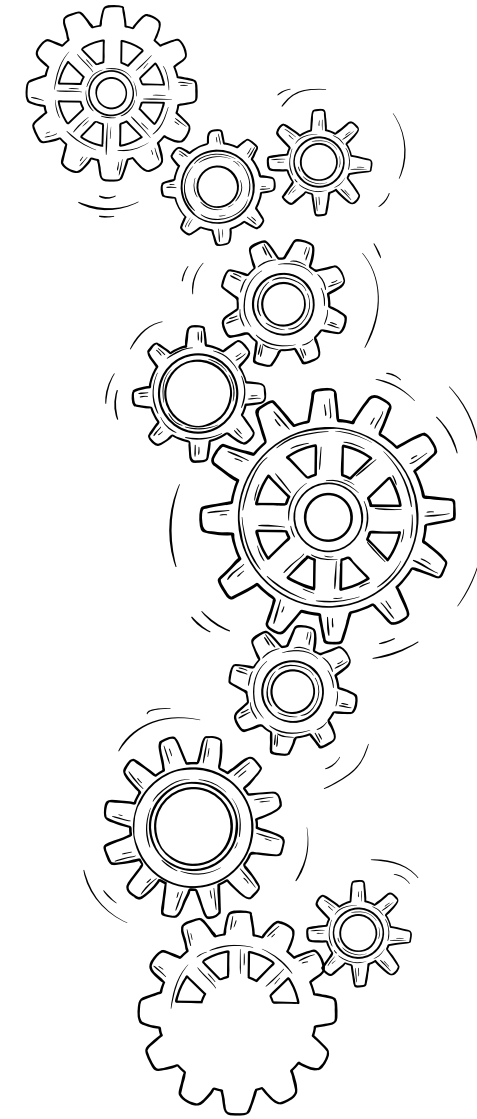
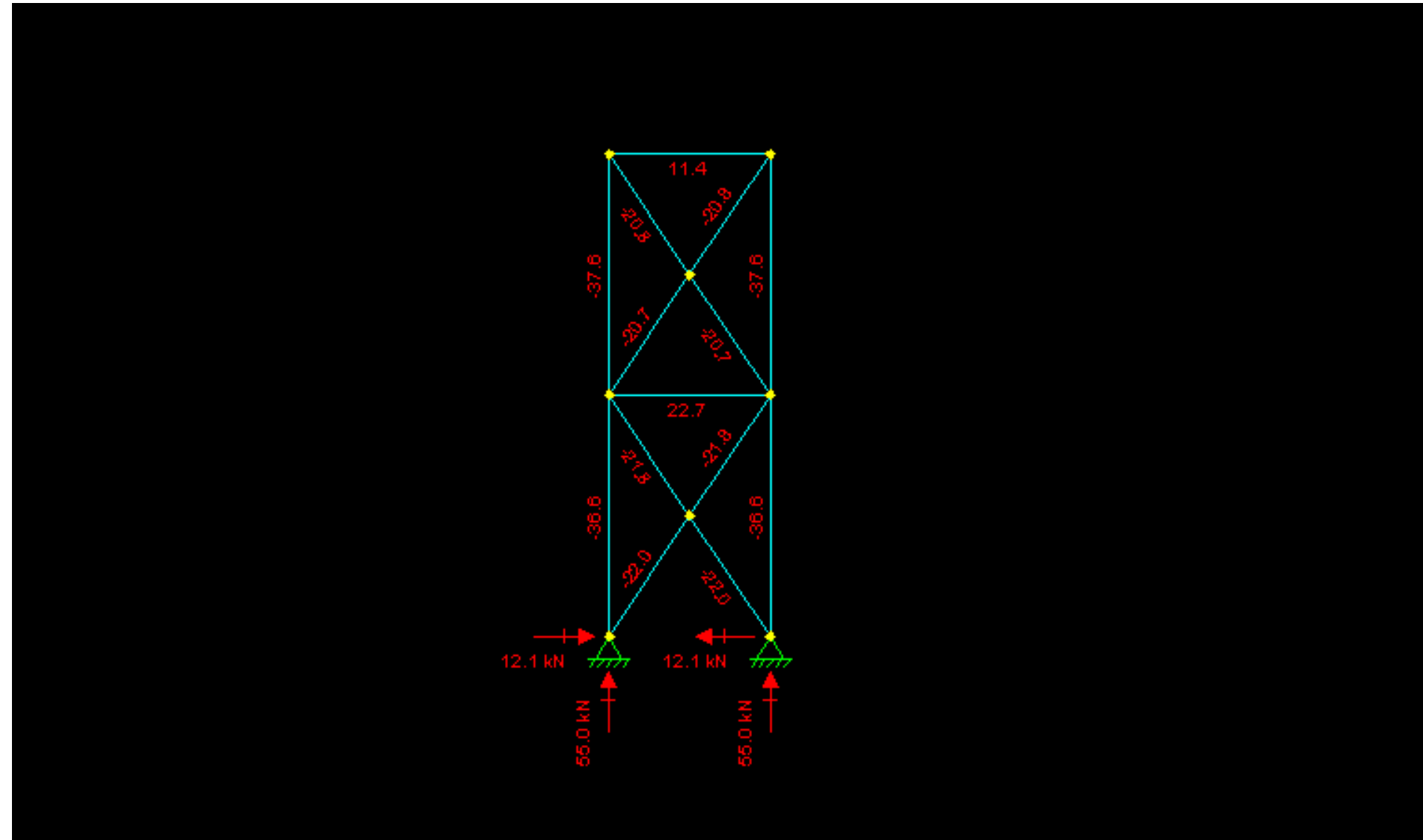
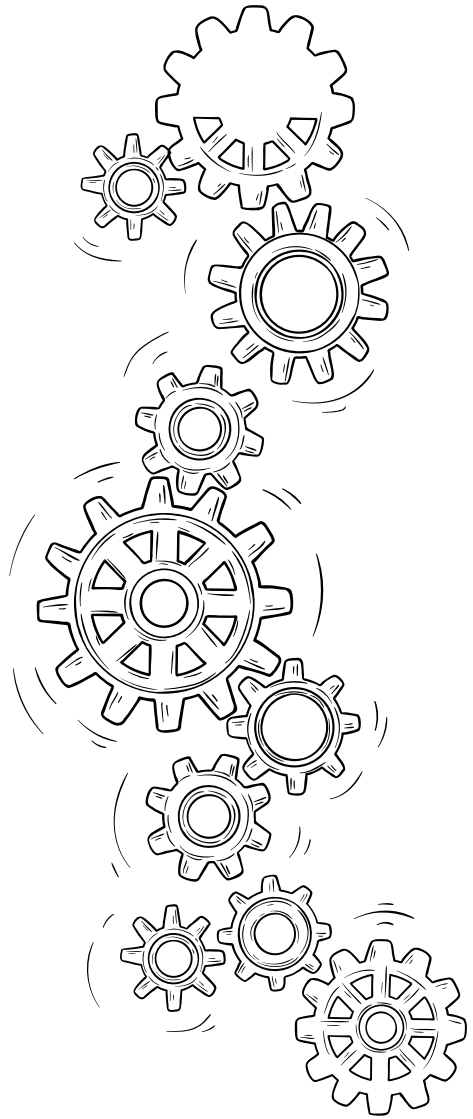
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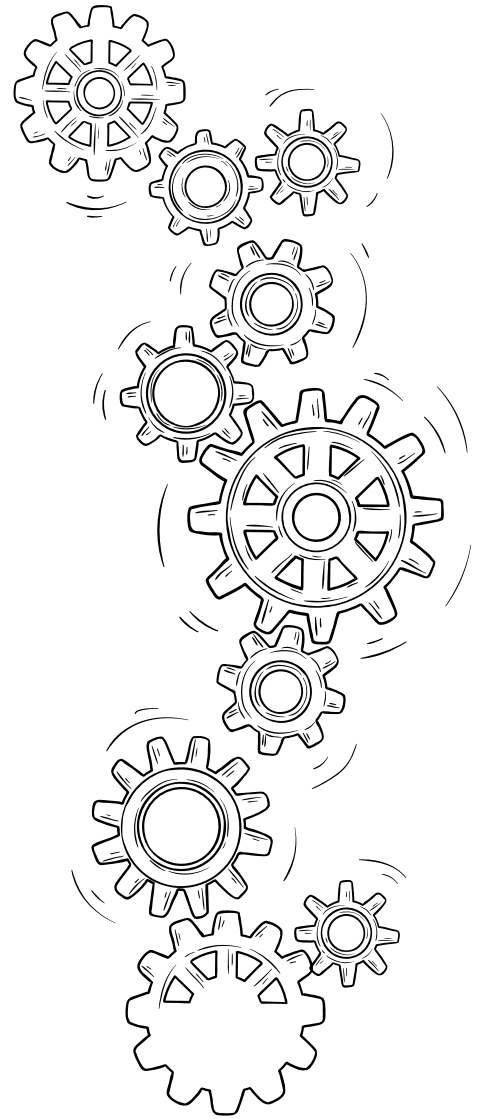
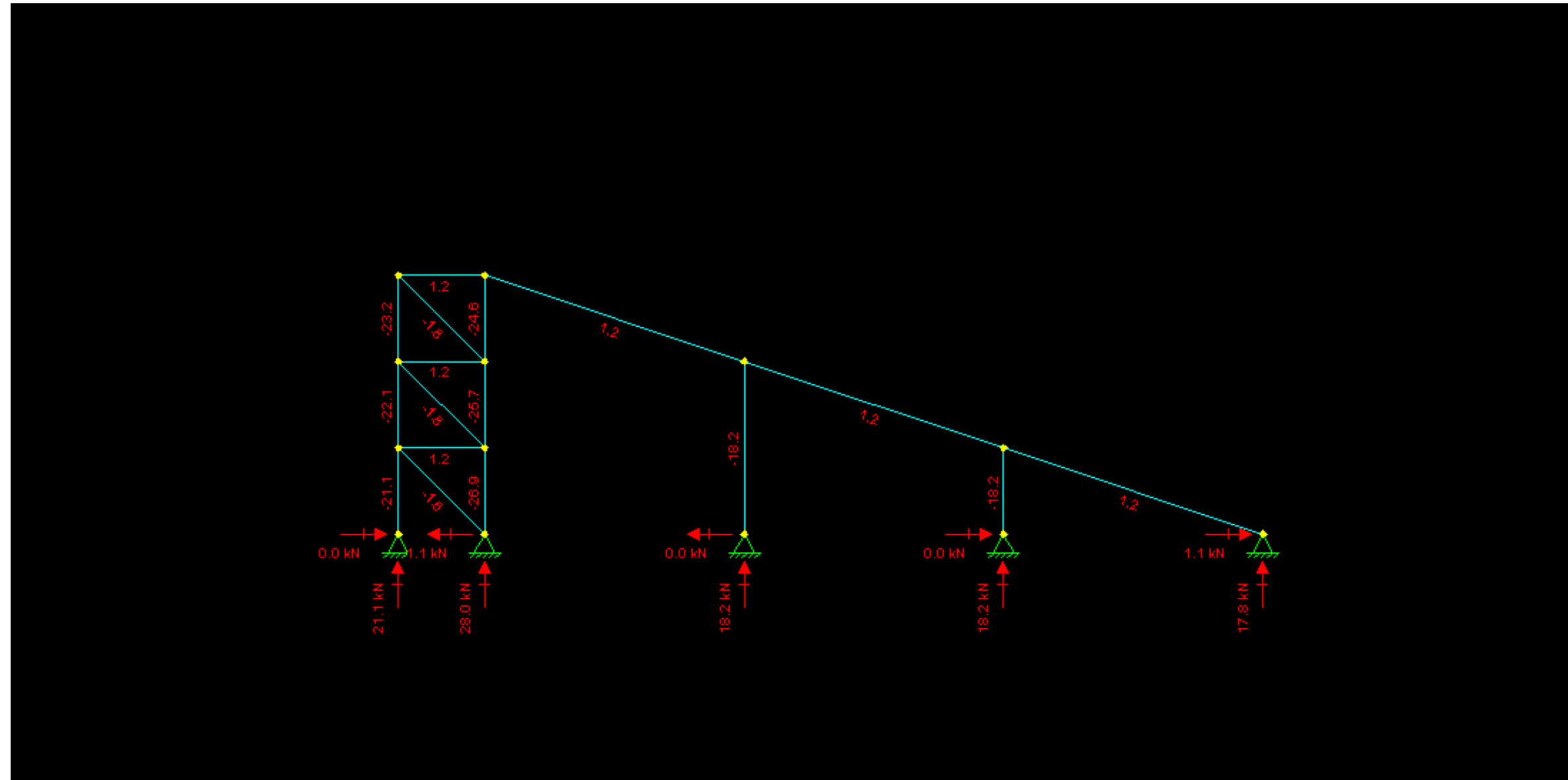
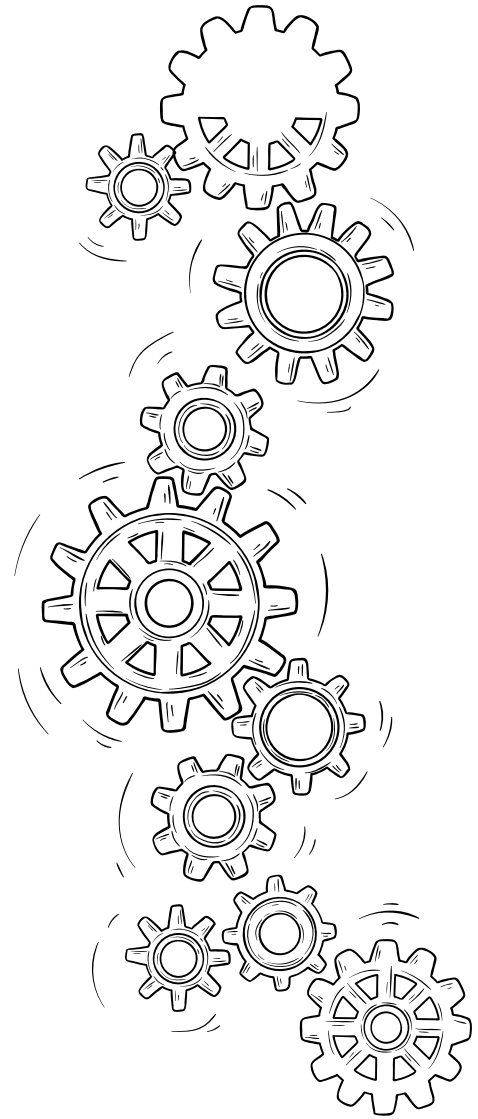
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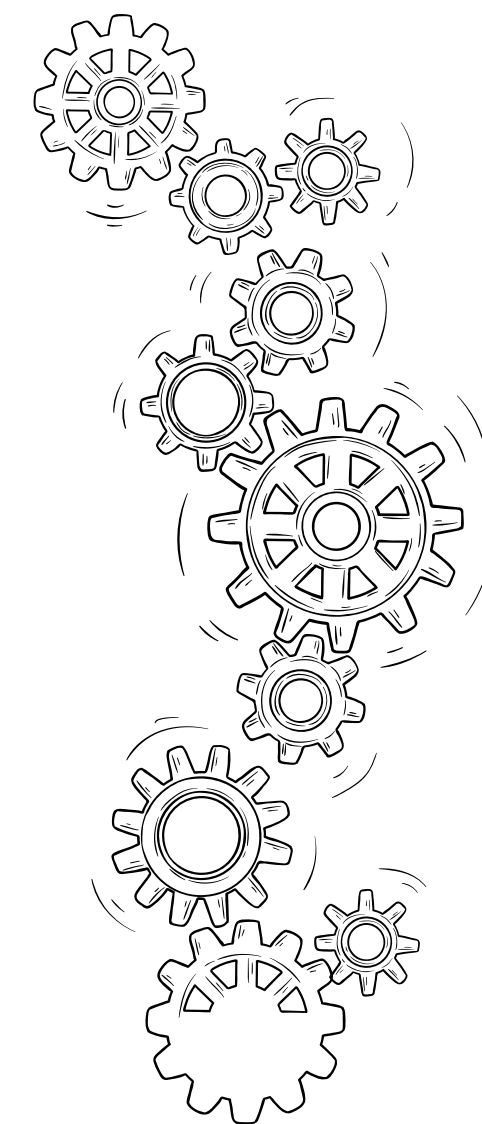
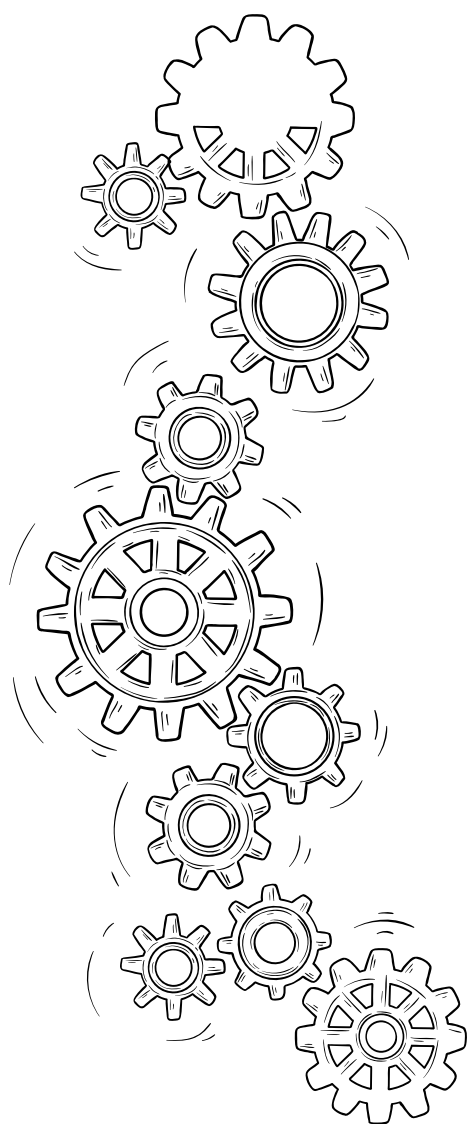


# Orçamento Simplificado

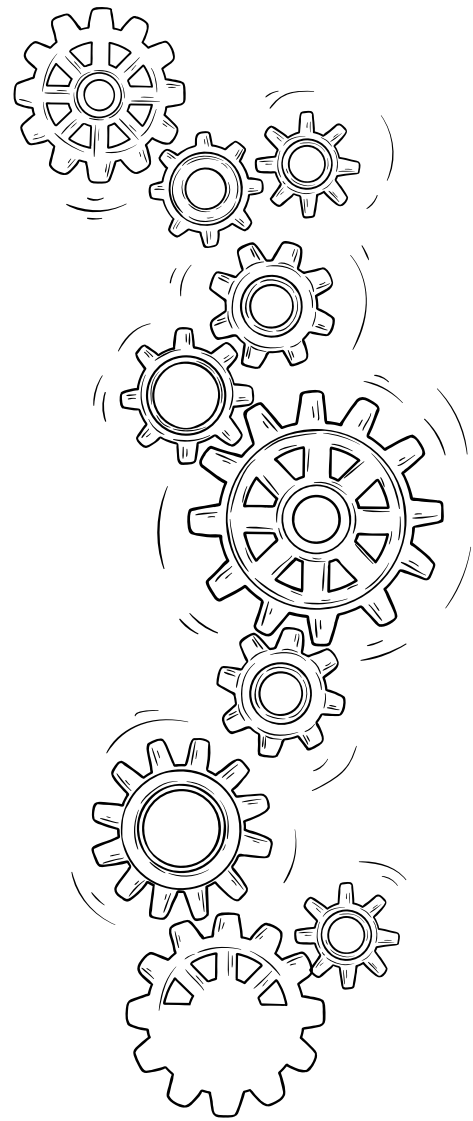
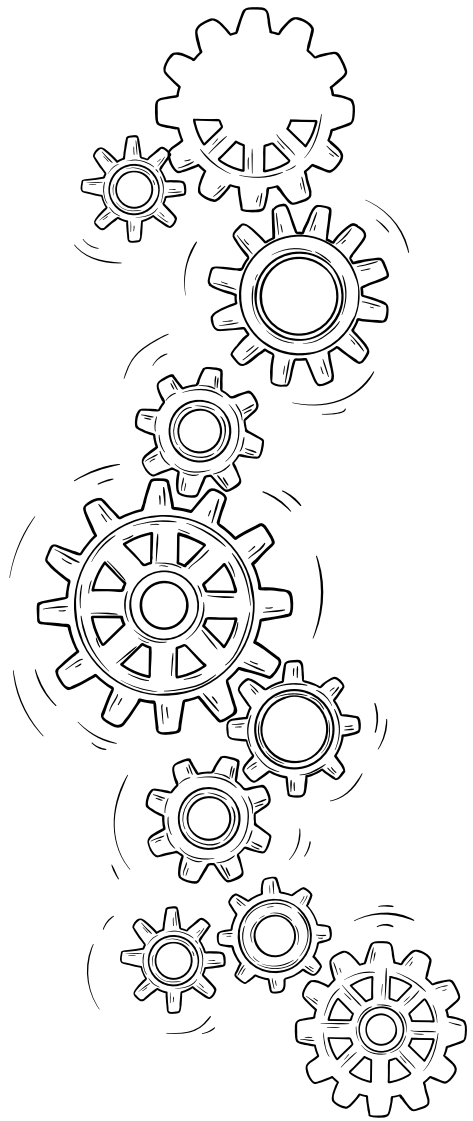
## ORÇAMENTO SIMPLIFICADO – PONTE ECOLÓGICA

Custo total estimado: R\$ 119.900,00 (109 m<sup>2</sup> × R\$ 1.100/m<sup>2</sup>)

Etapa	% do Total Valor (R\$)	Justificativa	
Serviços Preliminares e Gerais	4%	4.796,00	Limpeza, locação da obra, topografia básica
Fundação	7%	8.393,00	Fundação rasa com sapatas ou blocos simples
Estrutura metálica (Perfis H/I e Cant.)	40%	47.960,00	Principal componente – perfis, soldas, ferragens
Piso (chapa metálica com tela antidesslizante)	8%	9.592,00	Piso resistente ao tempo, com segurança para os animais
Fechamentos laterais (tela ou gradil)	6%	7.194,00	Contenção com visibilidade, evitando fuga/atropelamento
Cobertura parcial (opcional, tipo toldo)	5%	5.995,00	Proteção contra sol e chuva (caso necessário)
Acessos e Rampas	8%	9.592,00	Rampas de acesso em terra compactada ou gradil
Pintura Anticorrosiva e Sinalização	3%	3.597,00	Tinta de proteção + placas ecológicas
Instalações Hidráulicas (drenagem)	3%	3.597,00	Tubulações e calhas para escoamento pluvial
Instalações Elétricas (iluminação básica)	2%	2.398,00	Apenas se for necessário para segurança noturna
Limpeza final e Entrega Técnica	2%	2.398,00	Finalização, checagem e entrega
Imprevistos e Complementares	12%	14.388,00	Transporte, EPIs, aluguel de equipamentos, BDI



# Maquete



# Obrigado a todos!



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